

] 10 5r •

KAWASAKI STEEL GIHO

Vol.5 (1973) No.1

i)~ ) Å#Ý5δ'ö%® È -'Ä b " @3U, '0è9,

An Actual Loading Test of Steel Pipe Piled Well

% ¼ û G(Masao Ishiwata) ) i % ? # Œ(Masao Tominaga) /œ m (Hiroshi Yukitomo) % ¼#ä Å (Masahiro Ishida)

O[ " :

- c È å0 5r d \_ > 8 Z/œ ^ f € S i)~ ) Å#Ý5δ'ö%® È -'Ä b È ¹ • Š0è9,) Ý \+¬#ä ú ·0è9,) Ý † v \ \_5δ'ö%® È -'Ä b7ü\$× i ·\$x ö · † Å } ? \_ M • \ \ v \_0£ 2 b \ ö \_ X 8 Z è0! K>\* (1)4#5 - š ) - G3?0... b6ö € ? } š ) \_ P M •#/œ b0£ 2 @ ( ö ² [ 6 • G \>\* (2) Å Š0£'i \_ > 8 Z5δ'ö%® È -'Ä b [ ö † v W \9x C0Û o [ A • G \>\* #'/œ b Ç i³ å ç4Š b0£ c ? ^ ~ ö ² [ 6 • G \>\* (4) • [3Ù m S Ö ¹ Ý0Ž Ö c5δ'ö %® È -'Ä b ú ."l ö † š3Q\$×, C/² K : • G \>\* (5)5δ'ö%® È -'Ä c š3Q\$× ö/¶ ö @ ± A C>\* [ w i \_ X 8 Z v £ i ^ å x C 8 b p6ë\$× i †&g K Z 8 • G \^ ] @ Å } ? \_ ^ W S

## Synopsis :

This report explains static and dynamic behaviours of the Steel Pipe Piled Well, based on the results of the horizontal loading test and the free vibration test conducted to the Steel Pipe Piled Well constructed at the Mizushima Works. This report also examines the technical feasibility of design method, thereby verifying the following points: (1) The design method against deformation is sufficiently safe based on the interrelations among load, deformation and the angle of rotation. (2) In the stress calculation, the rigidity of the Steel Pipe Piled Well can be evaluated higher. (3) The design of the footing is fairly safe. (4) Model analysis described in this report expresses relatively well the vibration characteristics of the Steel Pipe Piled Well. (5) The Steel Pipe Piled Well has relatively large damping property, and the range of inherent values of it falls between two ranges of values in the respective cases of caisson and piling.

(c)JFE Steel Corporation, 2003

UDC 624.157.6 : 624.04

# 仮締切兼用鋼管矢板井筒の実物載荷試験

An Actual Loading Test of Steel Sheet Pile Wall

石渡 正夫\* 富永 真生\*\*

## 2. 設計概要

層の横抵抗はあまり期待できず、 $K_{H1}=0$  とし

よ、より安全側の八成七七四倍に土圧をもつ。

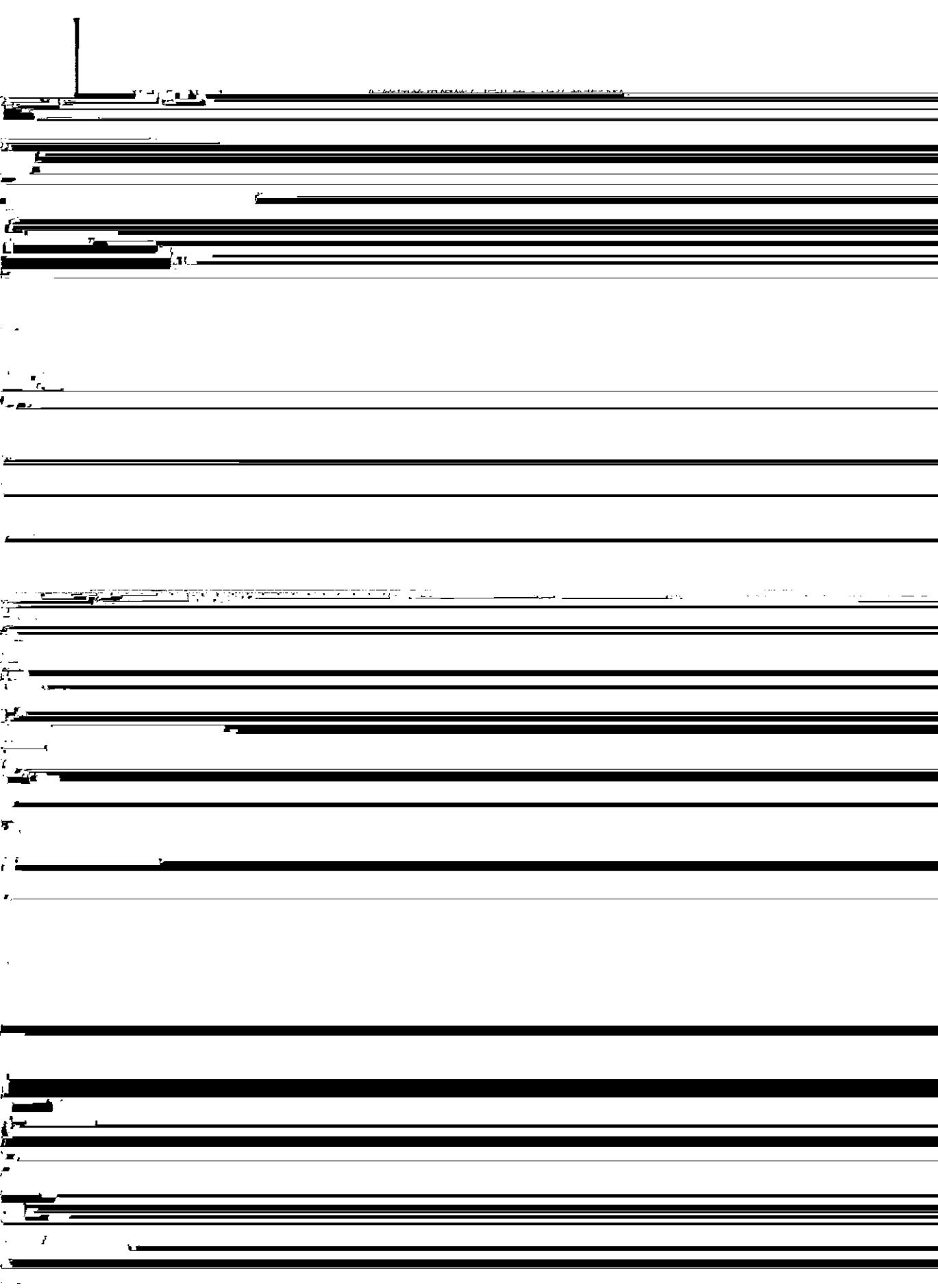
ここで述べる鋼管矢板井筒は港湾におけるドル

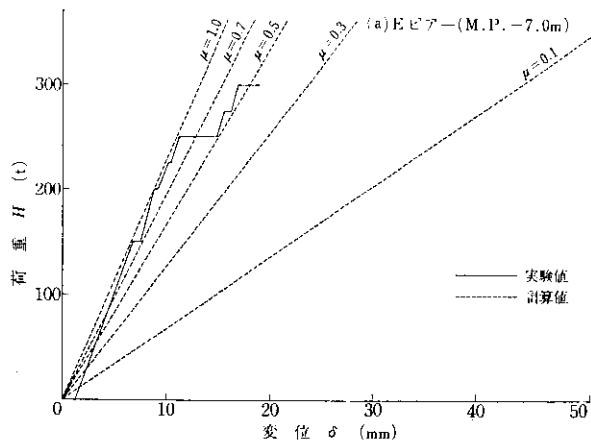
$K_V=2 \times K_{H3}$  とした。

れたEピア、Wピアに互いに反力をとらせな



+4.5mにおける両ピアの荷重—変位曲線を示す。残留変形量はEピアよりWピアの方が大きく現われているが、それを無視すれば両ピア～







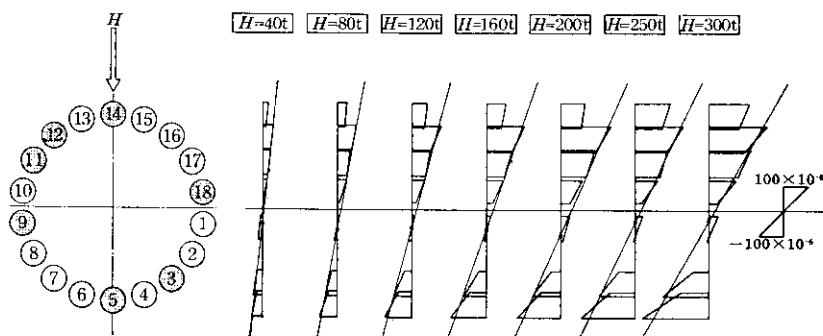


図 9 残留ひずみを含む鋼管矢板の曲げひずみ (M. P. - 12m)

### 3.2.3 フーチングの応力

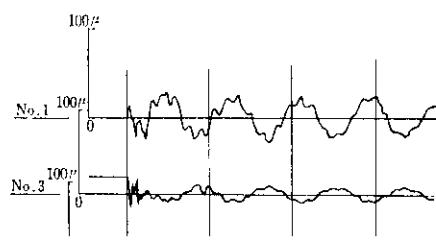
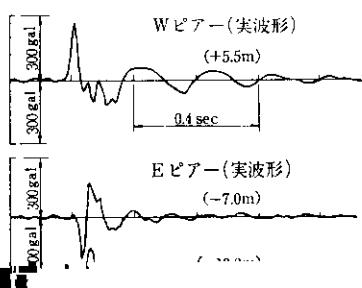
定する。鋼棒の一端に切断用鋼棒を接続し、他端には揚量 300 t の油圧ジャッキを設置する。静的



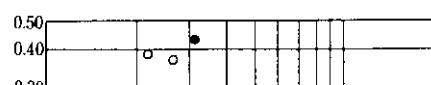
表 2 自由振動試験結果

(a) 最大加速度 (gal)





## 4.4.3 鋼管矢板井筒の減衰性



結果加速度の最大応答値はEピアで、530gal, W  
ピアで280galであった。ただし、実験場所が  
地盤各箇所の大小、会社十七ヶ所、十九ヶ所の

## 5. 結論

